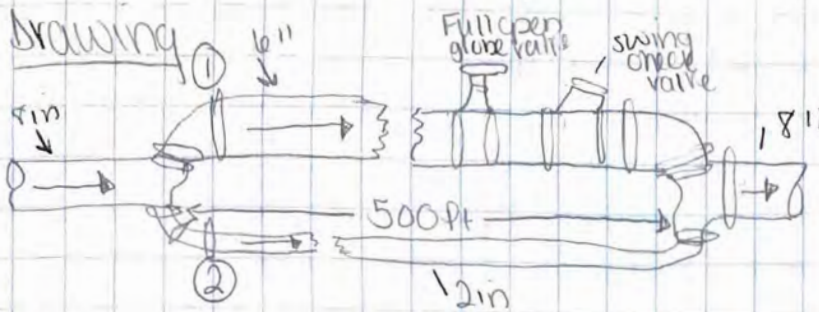


12-7

Purpose: Calculate the volume flow rate in the 6in and 2in pipes.



Materials:
benzene, steel

Design Considerations:

- ① Incompressible
- ② Isothermal
- ③ Steady state

Data:

$Sg_{\text{benzene}} = 0.87$ $Q = 1350 \text{ g/min} = 3.01 \text{ m}^3/\text{s}$

Fluid @ 140°F

Series 40 $D_{6"} = 2.067 \text{ in} = 0.1723 \text{ ft}$, $A_p = 0.02333 \text{ ft}^2$, $\epsilon = 1.5 \times 10^{-4} \text{ ft}$

$D_{2"} = 0.1665 \text{ in} = 0.01387 \text{ ft}$, $A_p = 0.0003 \text{ ft}^2$, $\epsilon = 1.5 \times 10^{-4} \text{ ft}$

$D_{8"} = 7.981 \text{ in} = 0.6651 \text{ ft}$, $A_p = 0.3472 \text{ ft}^2$, $\epsilon = 1.5 \times 10^{-4} \text{ ft}$

$N = 4.45 \times 10^{-6} \text{ ft}^2/\text{s}$ $f_{T(6)} = 0.015$ $f_{T(2)} = 0.019$

Procedure/Calculations: Express energy loss in each pipe: $h_f = K Q^2$

Branch 1 (6" pipe): $L = 500 \text{ ft}$

Branch 2: $L = 500 \text{ ft}$

(2) 90° elbows: $2 \times 30 \text{ ft} = 0.9$

(2) 90° elbows: $2 \times 30 \text{ ft} = 1.14$

(2) tees (flow thru): $2 \times 20 \text{ ft} = 0.6$

$\Sigma L = 1.14$

(1) Full open globe valve: $340 \text{ ft} = 5.1$

(1) Swing valve: $5 \text{ ft} = 0.75$

$\Sigma L = 7.35$

Solve by iteration: (Excel) (row 55)

Trial 0:

Branch	Pipe	$Q (\text{ft}^3/\text{s})$	$D (\text{ft})$	ϵ	L	L_e	D/ϵ	V
1	6 in	2.807	0.5054	1.50×10^{-4}	500 ft	7.35	3.37×10^3	13.9896
2	2 in	-0.201	0.1723	1.50×10^{-4}	500 ft	1.14	1.15×10^3	8.64197

(cont)

Re	f	K	$K \cdot Q^2$	$2 \cdot K \cdot Q$	ΔQ
1.59×10^6	1.55×10^{-2}	8.74	68.83345	49.05219	0.00
3.35×10^5	2.01×10^{-2}	1095.39	-0.88362	1083.2411	
		sum	0.00	732.2936	

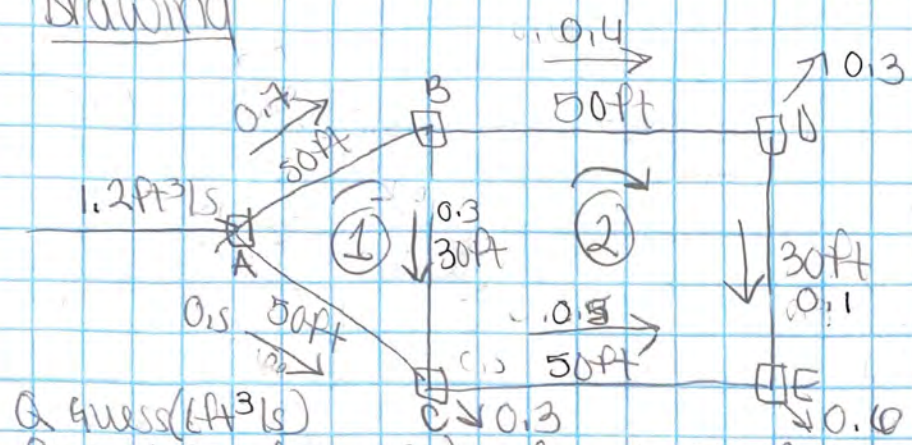
Summary: The flow rate for 6" pipe is $2.807 \text{ ft}^3/\text{s}$, and the flow rate for 2" pipe is $0.201 \text{ ft}^3/\text{s}$.

12-9

Purpose: Find the flow rate of water, at 100°F in each pipe

Materials: water, steel

Drawing



Design Considerations

- ① incompressible
- ② isothermal
- ③ steady state
- ④ neglect minor losses

Q guess (ft³/s)

Circuit one (Triangle)

- AB + 0.7
- BC + 0.3
- AC - 0.5

Circuit two (Rectangle)

- BD + 0.4
- DE + 0.1
- CE - 0.5
- BC - 0.3

Data:

- 2 1/2" sch 40:
- $D_i = 0.2058$ ft
- $A_f = 0.03326$ ft²
- $S_w = 62.4$ lb/ft³
- $\nu = 1.21 \times 10^{-5}$ ft²/s

Solve by Iteration (Hardy Cross) - Exce 1 (Row 101)

Trial	Circuit	Pipe	Q (ft³/s)	ΔQ
3	1	AB	0.5007	
		BC	0.5374	
		CA	-0.6993	-0.01
2	2	BD	0.5120	
		DE	0.2120	
		CE	-3.880	
		BC	-0.5374	-0.01

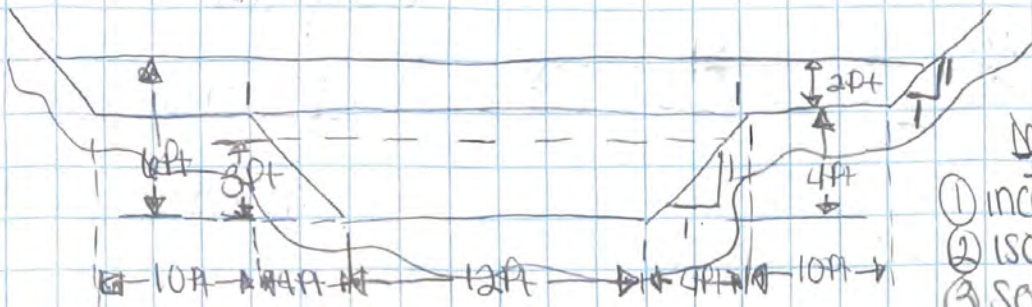
Summary:

In circuit 1 the Q per AB = 0.5007 ft³/s, BC = 0.5374 ft³/s, and CA = -0.6993 ft³/s.
 In circuit 2 the Q per BD = 0.5120 ft³/s, DE = 0.2120 ft³/s, CE = -3.880, and BC = -0.5374 ft³/s.

14.15 Purpose: Determine the normal discharge for depths 3ft + 6ft.

Drawing

Materials: earth/grass, water



Design

- ① incompressible
- ② isothermal
- ③ solid state

Data

$n = 0.04$
 slope' = 0.00015% NP = W + 2L
 $d_1 = 3\text{ft}$
 $d_2 = 6\text{ft}$

$A = WD + XD$ $R = \frac{A}{NP}$

Manning's Equation:

$Q = \frac{1.49}{n} A R_h^{2/3} S^{1/2}$

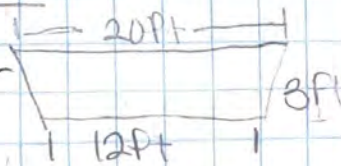
Procedure / Calculations

Ⓐ 3ft depth:

$A = (12+3) \times 3 = 45\text{ft}^2$

$NP = 12 + 2(3)\sqrt{2} = 20.49\text{ft}$

$R = \frac{45\text{ft}^2}{20.49\text{ft}} = 2.19\text{ft}$



$Q = \frac{1.49}{0.04} 45 \cdot (2.19)^{2/3} \cdot (0.00015)^{1/2}$

$Q = 34.684\text{ft}^3/\text{s}$

Ⓑ 6ft depth: Rect + Triangle

$A = (4 \times 2) + (\frac{1}{2} \times 2 \times 2) = 10\text{ft}^2$

$NP = 4 + \sqrt{2^2 + 2^2} = 6.83\text{ft}$

$R = \frac{10\text{ft}^2}{6.83\text{ft}} = 1.46\text{ft}$

$Q = \frac{1.49}{0.04} \cdot 10 \cdot (1.46)^{2/3} \cdot (0.00015)^{1/2}$

$Q = 5.87\text{ft}^3/\text{s} \cdot 2 = 11.74\text{ft}^3/\text{s}$

Lower trapezoid:

$A = (\frac{12+20}{2} \cdot 4) + (20(6-4)) = 104\text{ft}^2$

$Q = \frac{1.49}{0.04} \cdot 104 \cdot (4.46)^{2/3} \cdot (0.00015)^{1/2}$

$= 128.556\text{ft}^3/\text{s}$

$NP = 12 + 2 \cdot (4\sqrt{2}) = 23.314\text{ft}$

$R = \frac{104\text{ft}^2}{23.314\text{ft}} = 4.46\text{ft}$

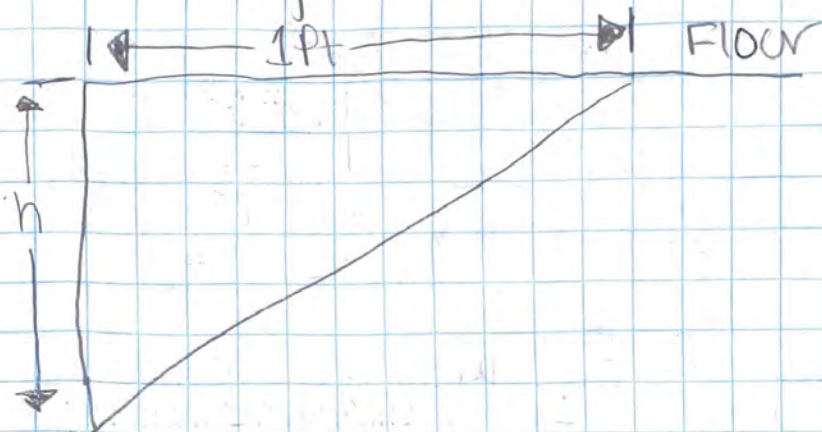
$\Sigma Q = 128.556 + 11.74 = 140.3\text{ft}^3/\text{s}$

Summary: The normal discharge at 3ft is 34.7 ft³/s, and at a depth of 6ft is 140.3 ft³/s.

14.21 Purpose: Determine the size of tile required to carry the flow (500 gal/min) when running half full

materials: water, clay tiles,

Drawing:



Design considerations:

- ① Incompressible fluid
- ② Isothermal
- ③ steady state

Data:

$$Q = 500 \text{ gal/min} = 1.114 \text{ ft}^3/\text{s}$$

$$s = 0.01\%$$

$$n = 0.013 \text{ (table 14.1)}$$

$$A = \frac{\pi D^2}{8}$$

$$R = \frac{D}{4}$$

Procedure / Calculations:

Manning's Equation

$$Q = \frac{1.49}{n} A R^{2/3} S^{1/2}$$

$$Q = \frac{1.49}{n} \cdot \left(\frac{\pi D^2}{8} \right) \cdot \left(\frac{D}{4} \right)^{2/3} \cdot S^{1/2}$$

$$Q = \left(\frac{1.49}{n} \cdot \frac{\pi}{8} \cdot \frac{1}{4} \right)^{2/3} \cdot \frac{1}{n} \cdot D^{2/3} \cdot S^{1/2}$$

$$Q = \frac{0.231}{n} \cdot D^{2/3} \cdot S^{1/2}$$

$$1.114 = \frac{0.231}{0.013} \cdot D^{2/3} \cdot (0.001)^{1/2}$$

$$1.114 = 0.5204 \cdot D^{2/3}$$

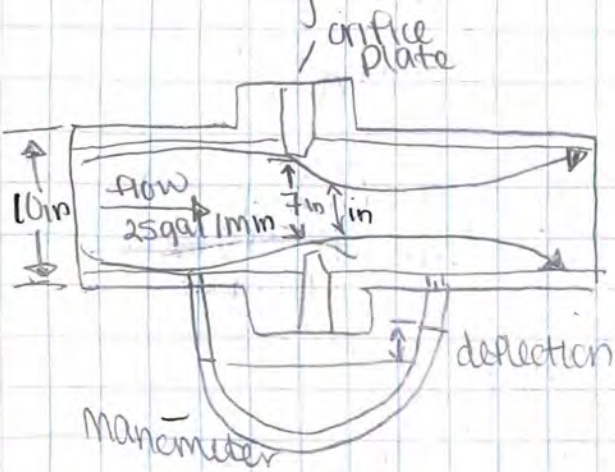
$$D^{2/3} = \frac{0.5204}{1.114} = 1.975$$

$$D = (1.975)^{3/2} = \boxed{1.29 \text{ ft}}$$

Summary: The diameter of the tile required to carry the 500 gal/min flow is 1.29 ft

15-4 Purpose: calculate the deflection of a water manometer (a) if orifice diameter is 1.0 in and (b) if the orifice diameter is 7 in.

Drawing:



materials: ammonia, water

Data:

$$d = 10 \text{ in} = 0.833 \text{ ft}, A = 0.5454 \text{ ft}^2$$

$$Q = 25 \text{ gal/min} = 0.0557 \text{ ft}^3/\text{s}$$

$$d_{\text{orif}} = 1 \text{ in} = 0.0833 \text{ ft}$$

$$d_{\text{orif}} = 7 \text{ in} = 0.5833 \text{ ft}$$

$$sg = 0.83$$

$$v = 2.5 \times 10^{-6} \text{ lb} \cdot \text{s} / \text{ft}^2$$

$$\rho_{\text{ammonia}} = 0.83 \times 1.94 \text{ slugs} / \text{ft}^3 = 1.61$$

$$SW_{\text{ammonia}} = 0.83 \times 62.5 = 51.792 \text{ lb} / \text{ft}^3$$

$$SW_{\text{water}} = 62.4 \text{ lb} / \text{ft}^3$$

Procedure / Calculations

(a) Calculate Re + Velocity

$$d = 1.0 \text{ in} \quad v_1 = \frac{Q}{A} = \frac{0.0557 \text{ ft}^3/\text{s}}{0.5454 \text{ ft}^2} = 0.1021 \text{ ft/s}$$

$$Re = \frac{\rho v d}{\mu} = \frac{(1.61)(0.1021)(0.833)}{2.5 \times 10^{-6} \text{ lb} \cdot \text{s} / \text{ft}^2} = 54,800$$

FR through orifice:

$$Q = C A_0 \sqrt{\frac{2g \Delta p}{\gamma_{\text{amm}}}} \quad A = \frac{\pi}{4} \left(\frac{1}{12} \text{ ft}\right)^2 = 0.00545 \text{ ft}^2 \quad C = 0.60$$

$$(A_0 A_1) = \beta^4 \quad \beta = \frac{1}{10} = 0.10$$

Pressure head

$$\frac{\Delta p}{\gamma_{\text{amm}}} = \frac{1 - \beta^4}{2g} \left(\frac{Q}{C A_0}\right)^2 = \frac{1 - (0.10)^4}{2(32.2)} \left(\frac{0.0557}{0.100 \cdot 0.00545}\right)^2 = 4.497$$

Deflection: $\Delta p = h (\gamma_{\text{water}} - \gamma_{\text{amm}}) \Rightarrow h = \frac{\Delta p}{\gamma_{\text{amm}}} \left(\frac{\gamma_{\text{amm}}}{\gamma_{\text{water}} - \gamma_{\text{amm}}}\right)$

$$h = 4.497 \left(\frac{51.792}{62.4 - 51.792}\right) = 21.95 \text{ ft or } 263.4 \text{ in}$$

(b) $d = 7.0 \text{ in} \quad \beta = \frac{7}{10} = 0.70 \quad A = \frac{\pi}{4} \left(\frac{7}{12}\right)^2 = 0.2673 \text{ ft}^2 \quad C = 0.60$

Pressure head:

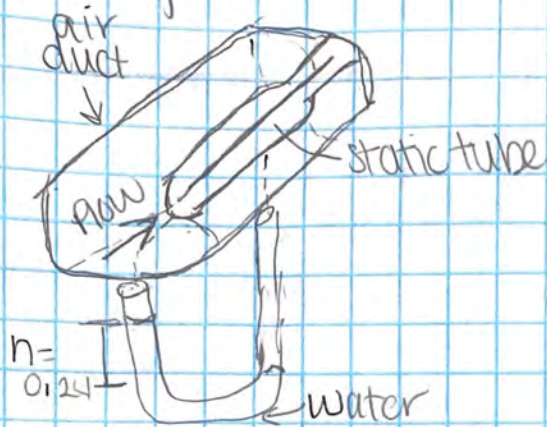
$$\frac{\Delta p}{\gamma_{\text{amm}}} = \frac{1 - (0.7)^4}{2(32.2)} \left(\frac{0.0557}{0.600 \times 0.2673}\right)^2 = 0.001378 \text{ ft ammonia}$$

Deflection: $h = 0.001378 \times \left(\frac{51.792}{62.4 - 51.792}\right) = 0.00683 \text{ ft} \times 12 = 0.081 \text{ in}$

Summary: With a 1 in orifice diameter the deflection should be 21.95 ft or 263.4 in. With a 7 in diameter orifice the deflection would be 0.00683 ft or 0.081 in.

15-15 Purpose: Calculate the velocity of flow.

Drawing:



Materials: water

Design Considerations

- 1) steady state
- 2) incompressible
- 3) isothermal

Data: $h = 0.24 \text{ in} = 0.02 \text{ ft}$
 $P_{\text{atm}} = 14.7 \text{ psi} = 2116.2 \text{ lb/ft}^2$
 $P_{\text{water}} = 62.4 \text{ lb/ft}^3$
 $S_{\text{water}} = 0.0765 \text{ lb/ft}^3$

Procedure Calculation:

Apply Bernoulli's Equation

$$\frac{P_1}{\gamma} + z_1 + \frac{v_1^2}{2g} = \frac{P_2}{\gamma} + z_2 + \frac{v_2^2}{2g}$$

$$\frac{P_{\text{stat}}}{\gamma} + \frac{v_1^2}{2g} = \frac{P_{\text{stag}}}{\gamma}$$

$$\Delta p = \gamma_a \left(\frac{v^2}{2g} \right)$$

$$P_{\text{stag}} - P_{\text{stat}} = h_w (\gamma_w - \gamma_a)$$

$$\Delta p = 0.02 \text{ ft} \times 62.4 \text{ lb/ft}^3 = 1.248 \text{ lb/ft}^2$$

Solve velocity:

$$v = \sqrt{\frac{2g(\Delta p)}{\gamma_a}} = \sqrt{\frac{2 \times 32.2 \times 1.248}{0.0765}} = \sqrt{1050.60} = 32.8 = \boxed{33.0 \text{ ft/s}}$$

Summary:

The velocity of the flow is 33 ft/s

Network Pipeline EXAMPLE Problem 1

TRIAL 1																			
Circuit	Pipe	Q (L/min)	Q (m³/s)	D (m)	e (ft)	L (ft)	Le (ft)	D/e	V (ft/s)	Re	f	k (s²/m⁴)	k*Q²	2*k*Q	Column1	Error %			
1	a	-1000	-0.0167	0.07384	1.50E-06	18.00	0.00	49226.67	3.8920	249901.77	0.015091	10224.974	-2.8403	340.8325		11.9%			
	b	500	0.0083	0.07384	1.50E-06	15.00	0.00	49226.67	1.9460	124950.89	0.017201	9711.7888	0.6744	151.8631		-23.7%			
	c	1000	0.0167	0.07384	1.50E-06	6.00	0.00	49226.67	3.8920	249901.77	0.015091	3408.3246	0.9468	113.6108	DeltaQ (m³/s)	-11.9%			
SUM														-1.2191	616.3064	-0.00198			
2	c	-1000	-0.0167	0.07384	1.50E-06	6.00	0.00	49226.67	3.8920	249901.77	0.015091	3408.3246	-0.9468	113.6108		19.7%			
	d	-2000	-0.0333	0.07384	1.50E-06	15.00	0.00	49226.67	7.7840	499803.55	0.013412	7572.567	-8.1440	504.5391		9.9%			
	e	1000	0.0167	0.07384	1.50E-06	15.00	0.00	49226.67	3.8920	249901.77	0.015091	8520.8114	2.3669	284.027		-19.7%			
	f	2000	0.0333	0.07384	1.50E-06	6.00	0.00	49226.67	7.7840	499803.55	0.013412	3029.0348	3.3656	201.9357	DeltaQ (m³/s)	-9.9%			
	SUM														-6.6283	1104.413	-0.00329		
3	f	-2000	-0.0333	0.07384	1.50E-06	6.00	0.00	49226.67	7.7840	499803.55	0.013412	3029.0348	-3.3656	201.9357		-22.9%			
	g	2000	0.0333	0.07384	1.50E-06	18.00	0.00	49226.67	7.7840	499803.55	0.013412	9687.1044	10.0968	695.807		22.9%			
	h	500	0.0083	0.07384	1.50E-06	15.00	0.00	49226.67	1.9460	124950.89	0.017201	9711.7888	0.6744	151.8631	DeltaQ (m³/s)	91.7%			
	SUM														7.4056	969.6058	0.00784		

Le (m) is for if you want to include minor losses

Calculate energy losses per branch:

$h_L = kQ^2$ where $k = 8(L + \sum Le)/\rho g D^5$

$v = 1.15E-06$

For kQ² we need to maintain pos/neg signs so formula ABS(k*Q²)

Compute Flow Rate Correction: DeltaQ = Sum(kQ²)/2*sum(2kQ)

TRIAL 2																				
Q(new) = Q (old) - Delta Q																				
Shared Branches: Q(new) - Q(old) - delta Q first + delta Q second																				
Circuit	Pipe	Q (L/min)	Q (m³/s)	D (m)	e (ft)	L (ft)	Le (ft)	D/e	V (ft/s)	Re	f	k (s²/m⁴)	k*Q²	2*k*Q	Column1	Error %				
1	a	-1000	-0.0141	0.07384	1.50E-06	18.00	0.00	49226.67	3.430107	2.20E+05	0.01544	10461.94	-2.25722	307.3429		5.06%				
	b	500	0.0103	0.07384	1.50E-06	15.00	0.00	49226.67	2.407629	1.55E+05	0.01650	8316.7291	0.990597	192.1367		-7.21%				
	c	1000	0.0154	0.07384	1.50E-06	6.00	0.00	49226.67	3.586768	2.30E+05	0.01532	3459.0176	0.816029	106.2574	DeltaQ (m³/s)	-4.84%				
SUM														1.45059	605.737	-0.00074				
2	c	-1000	-0.0154	0.07384	1.50E-06	6.00	0.00	49226.67	3.586768	2.30E+05	0.01532	3459.0176	-0.81603	106.2574		-6.72%				
	d	-2000	-0.0300	0.07384	1.50E-06	15.00	0.00	49226.67	7.016875	4.51E+05	0.01364	7700.8929	-6.95304	462.7944		-3.44%				
	e	1000	0.0200	0.07384	1.50E-06	15.00	0.00	49226.67	4.659197	2.99E+05	0.01482	8253.3448	3.285478	329.3399		5.17%				
	f	2000	0.0443	0.07384	1.50E-06	6.00	0.00	49226.67	10.3348	6.64E+05	0.01283	2987.8672	5.678229	256.4979	DeltaQ (m³/s)	2.33%				
	SUM														1.192293	1154.89	0.00100			
	g	-2000	-0.0443	0.07384	1.50E-06	6.00	0.00	49226.67	10.3348	6.64E+05	0.01283	2987.8672	-5.67583	256.4979		-1.75%				
3	h	2000	0.0257	0.07384	1.50E-06	15.00	0.00	49226.67	6.000467	3.85E+05	0.01400	8483.7625	6.261772	487.3814		3.01%				
	g	500	0.0007	0.07384	1.50E-06	18.00	0.00	49226.67	0.162431	1.04E+04	0.03065	13707.545	0.008374	24.07728	DeltaQ (m³/s)	111.26%				
	SUM														0.594316	767.9598	0.00077			

After 10 iterations the Answer is:

Pipe Branch	Flow rate (m³/s)	Flow rate (L/min)
a	0.0141	847.53
b	0.0109	652.47
c	0.0173	1037.27
d	0.0314	1884.79
e	0.0196	1115.21
f	0.0442	2653.81
g	0.0244	1461.90
h	0.0006	38.61

12-7 Homework

Trial 1																			
Branch	Pipe	Q (L/min)	Q (ft³/s)	D (ft)	e (ft)	L (ft)	Le (ft)	D/e	V (ft/s)	Re	f	k (s²/ft⁴)	k*Q²	2*k*Q	Column1	Qnew1	Qnew2		
1	6 in	675	1.504	0.5054	1.50E-04	500	7.35	3.37E+03	7.497053	8.51E+05	1.59E-02	8.89	20.98599	26.76966		2.248	1008.819		
	2 in	675	-1.504	0.1723	1.50E-04	500	1.14	1.15E+03	64.50457	2.50E+06	1.92E-02	1620.45	-3665.53	4874.336	Delta Q	-0.760	-341.181		
SUM														-3645.43	4901.062	0.743803619			

Trial 2																			
Branch	Pipe	Q (L/min)	Q (ft³/s)	D (ft)	e (ft)	L (ft)	Le (ft)	D/e	V (ft/s)	Re	f	k (s²/ft⁴)	k*Q²	2*k*Q	Column1	Qnew1	Qnew2		
1	6 in	1008.819064	2.248	0.5054	1.50E-04	500	7.35	3.37E+03	11.2047	1.27E+06	1.56E-02	8.78	44.37801	39.48547		2.604	1168.846		
	2 in	-341.1809357	-0.760	0.1723	1.50E-04	500	1.14	1.15E+03	32.60404	1.26E+06	1.93E-02	1633.54	-844.048	2483.66	Delta Q	-0.404	-181.154		
SUM														899.67	2523.146	0.356566819			

Trial 3																			
Branch	Pipe	Q (L/min)	Q (ft³/s)	D (ft)	e (ft)	L (ft)	Le (ft)	D/e	V (ft/s)	Re	f	k (s²/ft⁴)	k*Q²	2*k*Q	Column1	Qnew1	Qnew2		
1	6 in	1168.846253	2.604	0.5054	1.50E-04	500	7.35	3.37E+03	12.98208	1.47E+06	1.56E-02	8.75	58.36981	45.99226		2.757	1237.147		
	2 in	-181.1537471	-0.404	0.1723	1.50E-04	500	1.14	1.15E+03	17.31147	6.70E+05	1.96E-02	1654.85	-868.006	1335.044	Delta Q	-0.251	-112.853		
SUM														-210.247	1381.517	0.15			

Trial 4																			
Branch	Pipe	Q (L/min)	Q (ft³/s)	D (ft)	e (ft)	L (ft)	Le (ft)	D/e	V (ft/s)	Re	f	k (s²/ft⁴)	k*Q²	2*k*Q	Column1	Qnew1	Qnew2		
1	6 in	1237.147046	2.757	0.5054	1.50E-04	500	7.35	3.37E+03	13.74068	1.56E+06	1.56E-02	8.74	68.42974	48.18745		2.801	1257.144		
	2 in	-112.8529537	-0.251	0.1723	1.50E-04	500	1.14	1.15E+03	10.78449	4.18E+05	1.99E-02	1679.94	-106.222	844.8657	Delta Q	0.04	-0.207	-92.8557	
SUM														-39.7919	893.0541	0.04			

Trial 5																			
Branch	Pipe	Q (L/min)	Q (ft³/s)	D (ft)	e (ft)	L (ft)	Le (ft)	D/e	V (ft/s)	Re	f	k (s²/ft⁴)	k*Q²	2*k*Q	Column1	Qnew1	Qnew2		
1	6 in	1257.144284	2.801	0.5054	1.50E-04	500	7.35	3.37E+03	13.96278	1.59E+06	1.56E-02	8.74	68.57163	48.96009		2.806	1259.489		
	2 in	-92.85571835	-0.207	0.1723	1.50E-04	500	1.14	1.15E+03	8.873508	3.44E+05	2.01E-02	1693.40	-72.489	700.7231	Delta Q	-0.202	-90.5106		
SUM														-3.91739	749.6881	0.01			

Trial 6																			
Branch	Pipe	Q (L/min)	Q (ft³/s)	D (ft)	e (ft)	L (ft)	Le (ft)	D/e	V (ft/s)	Re	f	k (s²/ft⁴)	k*Q²	2*k*Q	Column1	Qnew1	Qnew2		
1	6 in	1259.56719	2.806	0.5054	1.50E-04	500	7.35	3.37E+03	13.98883	1.59E+06	1.56E-02	8.74	68.83345	49.05249		2.807	1259.567		
	2 in	-90.51056192	-0.202	0.1723	1.50E-04	500	1.14	1.15E+03	8.6494	3.35E+05	2.01E-02	1695.33	-68.862	683.8022	Delta Q	-0.201	-90.4328		
SUM														-0.12696	732.8517	0.00			

Trial 7																			
Branch	Pipe	Q (L/min)	Q (ft³/s)	D (ft)	e (ft)	L (ft)	Le (ft)	D/e	V (ft/s)	Re	f	k (s²/ft⁴)	k*Q²	2*k*Q	Column1	Qnew1	Qnew2		
1	6 in	1259.56719	2.807	0.5054	1.50E-04	500	7.35	3.37E+03	13.98969	1.59E+06	1.56E-02	8.74	68.83345	49.05249		2.807	1259.569		
	2 in	-90.43289962	-0.201	0.1723	1.50E-04	500	1.14	1.15E+03	8.6497	3.35E+05	2.01E-02	1695.39	-68.862	683.241	Delta Q	-0.201	-90.4311		
SUM														-0.00279	732.2936	0.00			

12-9 Homework:

Trial 1-																			
Circuit	Pipe	Q (L/min)	Q (ft³/s)	D (ft)	e (ft)	L (ft)	Le (ft)	D/e	V (ft/s)	Re	f	k (s²/ft⁴)	k*Q²	2*k*Q	Column1	Error %	Qnew		
1	AB	0.7000	0.2058	1.50E-06	50.00	0.00	137200.00	15.0531	25026.96	0.014889	0.014889	47.751549	23.3983	66.85217		-0.3%	0.60037		
	BC	0.3000	0.2058	1.50E-06	30.00	0.00	137200.00	9.0186	15391.27	0.016424	0.016424	33.584466	0.2029	20.15069		-0.7%	0.45566		
	AC	-0.5000	0.2058	1.50E-06	50.00	0.00	137200.00	15.0531	25565.21	0.014893	0.014893	50.775276	-12.6899	50.77573	DeltaQ (m³/s)	0.4%	-0.59963		
SUM														13.7282	137.787	0.10			
2	BD	0.4000	0.2058	1.50E-06	50.00	0.00	137200.00	12.0248	204521.69	0.015533	0.015533	52.957027	4.7371	42.86562		-0.8%	0.45602		
	DE	0.1000	0.2058	1.50E-06	30.00	0.00	137200.00	3.0062	51133.42	0.020687	0.020687	42.318119	0.2322	8.463632		-3			