

10-27-15

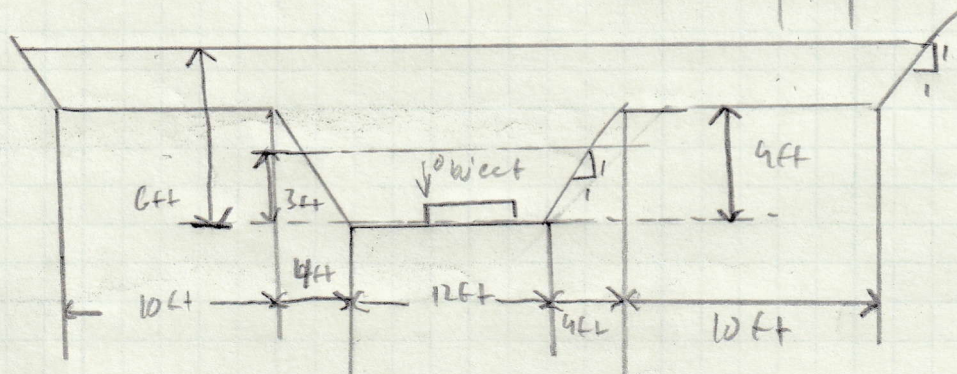
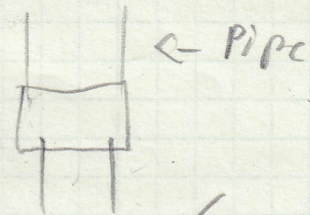
Test 1

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Purpose

A company wants me to design a system with a specific amount of requirements. This system delivers 60°F water from an open channel to another elevated one. This requires 30% of total fluid flowing in the lower open channel. This natural channel with light brush and its average being 0.00015. Each task has specific requirements.

- (a) Determine the required pump power (in HP) with a pump efficiency of 60%. Ignore minor losses and only consider pipe losses. Afterwards determine a pipe diameter.
- (b) How large should the buoy be for a gate to open when it reaches 38 inches? Is it stable?
- (c) What would be the reading in U-tube manometer.
- (d) From pump outlet to elevated open channel inlet, calculate the total horizontal and vertical forces in the pipe-elbows-valve system. Find the pump outlet pressure and elevated channel inlet pressure.
- (e) What is the pressure drop across the nozzle?
- (f) Find the occurrence of water hammer and/or cavitation. What is the pressure increment after the sudden closing? Would this pipe fail?
- (g) What is the object maximum weight so the fluid flow around it slides it along the bottom surface? Object does not tumble.



Sources

using the book

Matt, R, Unkenner J.A "Applied Fluid Mechanics" 7th edition
Pearson Education, Inc (2015)

Design Consideration

- Deliver 30% of total fluid flowing. Water $T = 60^{\circ}\text{F}$
- Slope average = 0.00015. ^(a) Neglect minor losses
- Neglect the weight of the circular gate and the buoy
- Discharge pipe is needed. - If pipe closes, suddenly
- Is there cavitation in the system - What if object is placed in the bottom channel
- Can the valve in the pipe fail?

Data and Variables

Water $T = 60^{\circ}\text{F}$

slope $S = 0.00015$

Total fluid = at least 30%

$\gamma_{\text{water}} = 62.4 \frac{\text{lb}}{\text{ft}^3}$

Water level = 38 in

Object is cylinder 5 ft / 1 foot side

Pipe length = 2500 ft

μ (Coeff of friction) = 0.30

$g = 32.2 \frac{\text{ft}}{\text{s}^2}$

diameter = 0.5

Materials

Water buoy in water tank

Pipes mercury in U-tube

Valves cylinder object

Flow nozzle

Procedure

a) We need to determine the pump power in hp

Before solving the problem, recognize what is given.

Pump efficiency of 60% = 0.60

Pipe length = 11 ft

Total discharge = 2500 ft

specific weight of water (γ) = 64.1 lb/ft³

From Table F.1

Schedule 40 Pipe 80

NPS = 3 in

DN = 80 mm

flow area = 0.05132 ft² or 4.72 x 10⁻³ m²

$$Q = A \sqrt{v^2}$$

$$V = \frac{Q}{A} = \frac{Q}{\frac{\pi}{4} D^2}$$

$$Q = (2 \text{ ft}^3/\text{s}) (0.02) = 0.06 \text{ ft}^3/\text{s}$$

$$V = 29.0 \text{ ft/s}$$

$$V = \frac{0.06 \text{ ft}^3/\text{s}}{0.05132 \text{ ft}^2}$$

$$z_2 = 40 \text{ ft}$$

$$z_1 = 10 \text{ ft}$$

$$f = 0.020$$

(friction)

We need to figure out the pump head in order to find pump power

$$\frac{p_1}{\gamma} + z_1 + \frac{V_1^2}{2g} + h_A - h_L = \frac{p_2}{\gamma} + z_2 + \frac{V_2^2}{2g}$$

$$\text{Pressure} = p_1 = p_2 = 0 \quad V_1 = V_2 = 0$$

$$z_1 + h_A - h_L = z_2$$

Recognize the friction loss in suction and discharge line, are minor losses. This equation solves for pipe loss

$$h_L = f \left(\frac{L}{D} \right) \left(\frac{V^2}{2g} \right) = (0.020)$$

$$= 0.020 \left(\frac{11 \text{ ft}}{0.05132 \text{ ft}} \right) \left(\frac{29 \text{ ft/s}}{2 (32.2 \text{ ft/s}^2)} \right)$$

$$= 1.930 \text{ ft}$$

Power equation

$$P = \frac{h_A \gamma Q}{e_m}$$

Pump head

$$h_A = z_2 - z_1 + h_L$$

$$= (40 \text{ ft} - 10 \text{ ft}) + 1.930 \text{ ft}$$

$$h_A = 31.93 \text{ ft}$$

$$= (31.93 \text{ ft}) (62.4 \text{ lb/ft}^3) (0.06 \text{ ft}^3/\text{s})$$

$$= 119.24 \text{ ft} \cdot \text{lb/s} \left(\frac{1}{550 \text{ ft} \cdot \text{lb/s}} \right)$$

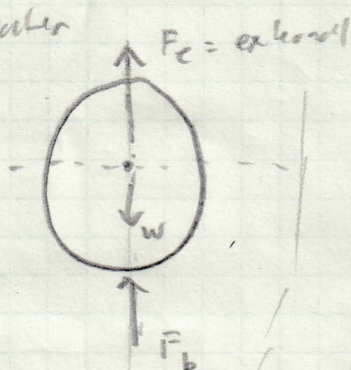
$$\text{Pump power} = 0.36 \text{ hp}$$

Procedure

(b) How large is the buoy to open the gate when the it reaches 39 inches.

The buoyant force is $F_b = \gamma V_d$

In water



Resultant force = $F_R = \gamma h_c A$

Force acting on buoy as it sinks

$$F_b + F_c - W = 0$$

$$F_c = -F_b$$

Neglect the weight and circular gate weight

Vertical height of tank

$$h_c = 39 + \cos 30^\circ = 39.87 \text{ in}$$

Area of the gate

$$A = \frac{\pi}{4} D^2 = \frac{\pi 10^2}{4} = 78.54 \text{ in}^2$$

Now we have the area,

We can find the resultant force

$$F_R = \gamma h_c A$$

$$= (62.4 \frac{\text{lb}}{\text{ft}^3}) \left(\frac{39.87 \text{ in}}{12} \right) \left(\frac{78.54 \text{ in}^2}{12^2} \right)$$

$$= (62.4 \frac{\text{lb}}{\text{ft}^3}) (3.239 \text{ ft}) (0.5454 \text{ ft}^2)$$

$$F_R = 110.2 \text{ lb}$$

← How big the buoy would be

Inertia of circular gate

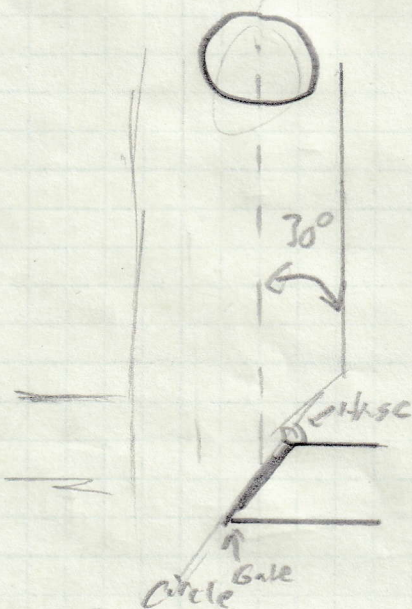
we need this when gate is pulled

$$I_c = \frac{\pi D^4}{64} = \frac{\pi 10^4}{64} = 496.87 \text{ in}^4$$

reference axis to centroid

$$L_c = \frac{h_c}{\cos 30^\circ} = \frac{39.87 \text{ in}}{\cos 30^\circ}$$

$$L_c = 44.98 \text{ in}$$



Continued from (b)

$$L_p - L_c = \frac{I_c}{L_c A} \quad \text{transfer theorem}$$

We need to find $L_p - L_c$ which is just the reference axis to the location of resultant force plus values in

$$L_p - L_c = \frac{I_c}{L_c A} = \frac{490.87 \text{ in}^4}{48.98 \text{ in} (78.51 \text{ in}^2)}$$

$$L_p - L_c = 0.12786 \approx 0.128 \text{ in}$$

In order to find out if the buoy is stable, the meta center must be greater than the center of buoyancy, $y_{mc} > y_{cb}$ (stable)

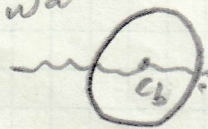
$$y_{mc} = I_c = 490.87 \text{ in}^4 = 3.409 \text{ ft}^2$$

Center of gravity is equal to

$$y_{cb} = \frac{L_c}{2} = \frac{22.44 \text{ in}}{12} = 1.87 \text{ ft}$$

$$3.409 \text{ ft}^2 > 1.87 \text{ ft}$$

water



The buoy is stable because $y_{mc} > y_{cb}$

(C) U-tube manometer. Find energy losses

$K = 520$ Pressure resistance



Mercury
 $sg = 13.54$

Computing losses

$$\frac{\Delta p}{\gamma} = K \frac{V^2}{2g}$$

Energy loss

$$h_L = 5.30 \left(\frac{V^2}{2(32.2 \text{ ft/s}^2)} \right)$$

Find V^2 Based on Schedule 40 Pipe 80

$$V = \frac{Q}{A} = \frac{0.106 \text{ ft}^3/\text{s}}{\frac{\pi}{4} \left(\frac{20 \text{ in}}{12} \right)^2}$$

$$V = 29.0 \text{ ft/s}$$

$$h_L = 5.30 \left(\frac{29.0^2}{2(32.2 \text{ ft/s}^2)} \right)$$

$$h_L = 2.387 \text{ ft/s} \text{ per energy loss in the tube}$$

with the mercury inside the tube that pressure

is

$$P_0 = \gamma h$$

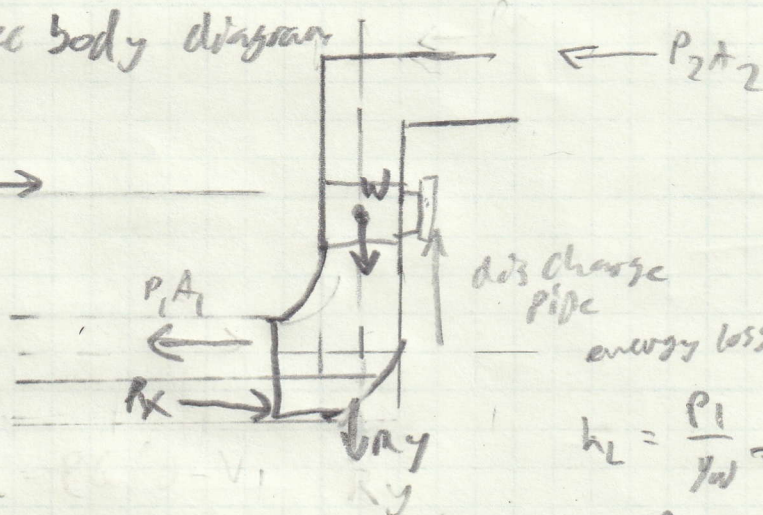
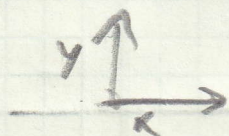
$$= 13.54 (20 \text{ inches})$$

$$P_0 = \frac{270.8 \text{ in}}{12}$$

$$P_0 = 22.56$$

(d) Find the supports total horizontal and vertical
start with

free body diagram



Find forces

$$\sum F_x = 0$$

$$-P_1 A_1 + R_x = -P_2 A_2$$

$$R_x = -P_1 A_1 - P_2 A_2$$

$$R_x = (120.49 \text{ lb/ft}^2)(0.5457 \text{ ft}^2)$$

$$R_x = 65.75 \text{ lb}$$

$$\sum F_y = 0$$

$$-R_y = -W \quad R_y = W$$

Weight of pipe

$$W = \gamma_w \left(\frac{\pi}{4} D^2 \right) (L)$$

$$W = (62.43 \text{ lb/ft}^3) \left(0.05132 \text{ ft}^2 \right) \left(\frac{20 \text{ ft}}{12} \right)$$

$$W = 5.340 \text{ lb}$$

$$R_y = 5.340 \text{ lb}$$

energy loss = $\frac{\text{Pressure}}{\text{Sp. wt. of water}}$

$$h_L = \frac{P_1}{\gamma_w} = 62.43 \text{ lb/ft}^3$$

$$P_1 = h_L \gamma_w$$

$$P_1 = (1.93044)(62.43 \text{ lb/ft}^3)$$

$$P_1 = 120.49 \text{ lb/ft}^2$$

$$A_1 = \frac{78.54 \text{ in}^2}{144} = 0.5457 \text{ ft}^2$$

based on previous problem

e) Determine pressure drop across nozzle

Flow meter equation ratio. $\frac{D}{d} = 0.5$

$$Q = C A_1 \sqrt{\frac{2g \frac{\Delta P}{\gamma_w}}{\left(\frac{A_1}{A_2}\right)^2 - 1}}$$

Find change of pressure based on the following

$$\frac{A_1}{A_2} = \left(\frac{D}{d}\right)^2 = (0.5)^2 = 0.25$$

Finding the flow discharge based on Table 15.3

$C = 0.984$ Just chose one

Have all variables except ΔP

Find change in pressure

$$\Delta P = \frac{\left(\frac{Q}{C A_1}\right)^2 \left(\left(\frac{A_1}{A_2}\right)^2 - 1\right) \gamma_w}{2g} = \frac{\left(\frac{0.106 \text{ ft}^3/\text{s}}{(0.984)(0.082 \text{ ft})}\right)^2 (16 - 1) (62.4 \text{ lb/ft}^3)}{2 (32.2 \text{ ft/s}^2)}$$

$$\Delta P = 20.52 \text{ lb}$$

This is the change of pressure drop across the nozzle

f) Basic Wall Thickness

$$t = \frac{PD}{2(SB + PY)} \quad \text{must use to find thickness}$$

First find water hammer and/or cavitation
This will help ensure the pipes being used are safe.

Cavitation

$$C = \frac{\sqrt{\frac{E_0}{P_w}}}{\sqrt{1 + \frac{E_0}{E_y}}}$$

$$C = \frac{\sqrt{\frac{200 \times 10^9 \text{ Pa}}{1000}}}{\sqrt{1 + \frac{200 \times 10^9 \text{ Pa}}{(2179 \times 10^6)(4.768 \times 10^{-3} \text{ ft}^2)}}$$

$$C = \frac{14142 \frac{\text{ft}}{\text{s}}}{139.75 \text{ ft}}$$

$$C = 101.93 \frac{\text{ft}}{\text{s}}$$

Wave velocity
for this extension

$$E_0 = 200 \text{ GPa}$$

Values for bulk modulus
for water Table 1.3

$$E = 2179 \text{ MPa}$$

$$y = 4.768 \times 10^{-3} \text{ ft}^2$$

Flow rate

$$P_w = 1000$$

Pressure water

Summary

This whole system is a combination of different pipes and tanks connected. Water is flowing through this system to an open channel. The system is all based on the pipes I selected in Table P.1.

The idea was to inspect and determine what was best for this system. Calculations that were used were meant to showcase the possible results.

Analysis

I think, the system could be